No.: 1/2020.

Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $\mathrm{c}_{\mathrm{s}}=80 \%$ spare-value) as a dependent of speed (velocity, v ) in a range of $0-120 \mathrm{~km} / \mathrm{h}$ with intervals of $5-5 \mathrm{~km} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\text {max }}$ ) and the optimal speed ( $\mathrm{v}_{\text {opt }}$ ) considering the following average paramameter values ( $\mathrm{g}=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $I_{\text {veh }}$ | $=$ | 6,2 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :--- | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,6 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1,2 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,65 | $[-]$ friction factor |
| $\mathrm{q}_{\mathrm{s}}$ | $=$ | $-0,2$ | $[\%]$ slope $(+$ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,5 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE* | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 7365 | 1,0 |  |
| light truck | 1512 | 1,4 |  |
| heavy truck | 1171 | 1,8 |  |
| trailer truck | 60 | 2,5 |  |
| non-articulated bus | 261 | 1,8 |  |
| articulated bus | 170 | 2,5 |  |
| motorcycle | 140 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is $11 \%(\omega)$ of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $\mathrm{v}_{\mathrm{s}}$, speed of traffic flow) is $39,6 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity $\left(\mathrm{c}_{\mathrm{a}}\right)$ |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $37,9 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by - $4,8 \%$ simultaneously.

If so calculate the new capacity utilisation as well.

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $\mathrm{c}_{\mathrm{s}}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $0-120 \mathrm{~km} / \mathrm{h}$ with intervals of $5-5 \mathrm{~km} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\text {max }}$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $g=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 5,4 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :--- | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,5 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1,1 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,78 | $[-]$ friction factor |
| $\mathrm{q}_{\mathrm{s}}$ | $=$ | $-2,7$ | $[\%]$ slope $(+$ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,5 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE $^{*}$ | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 6722 | 1,0 |  |
| light truck | 2444 | 1,4 |  |
| heavy truck | 817 | 1,8 |  |
| trailer truck | 235 | 2,5 |  |
| non-articulated bus | 255 | 1,8 |  |
| articulated bus | 162 | 2,5 |  |
| motorcycle | 64 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is $9,1 \%(\omega)$ of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $\mathrm{v}_{\mathrm{s}}$, speed of traffic flow) is $38,7 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity ( $\mathrm{c}_{\mathrm{a}}$ ) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $32,5 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by $-8,3 \%$ simultaneously.

If so calculate the new capacity utilisation as well.

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $c_{s}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $\mathbf{0 - 1 2 0} \mathbf{~ k m} / \mathrm{h}$ with intervals of $\mathbf{5 - 5} \mathbf{~ k m} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\max }$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $g=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 6 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :--- | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,6 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1,1 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,59 | $[-]$ friction factor |
| $\mathrm{q}_{s}$ | $=$ | $-1,3$ | $[\%]$ slope (+ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,5 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE* | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 12453 | 1,0 |  |
| light truck | 1341 | 1,4 |  |
| heavy truck | 1700 | 1,8 |  |
| trailer truck | 78 | 2,5 |  |
| non-articulated bus | 106 | 1,8 |  |
| articulated bus | 62 | 2,5 |  |
| motorcycle | 145 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is $12 \%(\omega)$ of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $v_{s}$, speed of traffic flow) is $42,3 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity (ca) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $35,8 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by -10,6 \% simultaneously.

If so calculate the new capacity utilisation as well.

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $c_{s}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $\mathbf{0 - 1 2 0} \mathbf{~ k m} / \mathrm{h}$ with intervals of $\mathbf{5 - 5} \mathbf{~ k m} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\max }$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $g=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 6,4 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :--- | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,5 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1,1 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,77 | $[-]$ friction factor |
| $\mathrm{q}_{s}$ | $=$ | 1,1 | $[\%]$ slope (+ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,6 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE* | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 7611 | 1,0 |  |
| light truck | 1444 | 1,4 |  |
| heavy truck | 1611 | 1,8 |  |
| trailer truck | 123 | 2,5 |  |
| non-articulated bus | 339 | 1,8 |  |
| articulated bus | 115 | 2,5 |  |
| motorcycle | 85 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is 9,9 \% ( $\omega$ ) of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $v_{s}$, speed of traffic flow) is $57,4 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity ( $\mathrm{c}_{\mathrm{a}}$ ) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $56,3 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by $+21,8 \%$ simultaneously.

If so calculate the new capacity utilisation as well.

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $\mathrm{c}_{\mathrm{s}}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $0-120 \mathrm{~km} / \mathrm{h}$ with intervals of $5-5 \mathrm{~km} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\text {max }}$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $g=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 5,6 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :---: | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,7 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,57 | $[-]$ friction factor |
| $\mathrm{q}_{\mathrm{s}}$ | $=$ | $-1,1$ | $[\%]$ slope $(+$ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,6 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE $^{*}$ | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 7365 | 1,0 |  |
| light truck | 2023 | 1,4 |  |
| heavy truck | 1171 | 1,8 |  |
| trailer truck | 79 | 2,5 |  |
| non-articulated bus | 295 | 1,8 |  |
| articulated bus | 29 | 2,5 |  |
| motorcycle | 178 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is $10,8 \%(\omega)$ of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $\mathrm{v}_{\mathrm{s}}$, speed of traffic flow) is $40,9 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity ( $\mathrm{c}_{\mathrm{a}}$ ) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $39,3 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by $+16,1 \%$ simultaneously.

If so calculate the new capacity utilisation as well.

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $c_{s}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $\mathbf{0 - 1 2 0} \mathbf{~ k m} / \mathrm{h}$ with intervals of $\mathbf{5 - 5} \mathbf{~ k m} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\max }$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $\mathrm{g}=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 6,3 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :---: | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,7 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,63 | $[-]$ friction factor |
| $\mathrm{q}_{s}$ | $=$ | 2,7 | $[\%]$ slope (+ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,5 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE $^{*}$ | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 14056 | 1,0 |  |
| light truck | 2222 | 1,4 |  |
| heavy truck | 1539 | 1,8 |  |
| trailer truck | 279 | 2,5 |  |
| non-articulated bus | 428 | 1,8 |  |
| articulated bus | 120 | 2,5 |  |
| motorcycle | 231 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is $12,7 \%(\omega)$ of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $v_{s}$, speed of traffic flow) is $59 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity (ca) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $59,7 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by -2 \% simultaneously.

If so calculate the new capacity utilisation as well.

No.: 7/2020. Name/Neptun: Matalqah Issa Mamoun Saleh/F1R6LA

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $c_{s}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $\mathbf{0 - 1 2 0} \mathbf{~ k m} / \mathrm{h}$ with intervals of $\mathbf{5 - 5} \mathbf{~ k m} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\max }$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $\mathrm{g}=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 6,3 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :---: | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,7 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,69 | $[-]$ friction factor |
| $\mathrm{q}_{s}$ | $=$ | $-0,4$ | $[\%]$ slope (+ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,5 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE* | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 9389 | 1,0 |  |
| light truck | 2889 | 1,4 |  |
| heavy truck | 1503 | 1,8 |  |
| trailer truck | 78 | 2,5 |  |
| non-articulated bus | 428 | 1,8 |  |
| articulated bus | 135 | 2,5 |  |
| motorcycle | 78 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is $12,2 \%(\omega)$ of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $v_{s}$, speed of traffic flow) is $63 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity (ca) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $62,4 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by $+5,5 \%$ simultaneously.

If so calculate the new capacity utilisation as well.

No.: 8/2020. Name/Neptun: Alatawneh Anas Abdullah Ahmad/H6ATDS

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $c_{s}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $\mathbf{0 - 1 2 0} \mathbf{~ k m} / \mathrm{h}$ with intervals of $\mathbf{5 - 5} \mathbf{~ k m} / \mathbf{h}$. Afterwards calculate the maximum flow (capacity, $q_{\max }$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $g=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 6,3 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :--- | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,6 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1,2 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,62 | $[-]$ friction factor |
| $\mathrm{q}_{s}$ | $=$ | $-1,9$ | $[\%]$ slope (+ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,5 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE $^{*}$ | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 13468 | 1,0 |  |
| light truck | 1541 | 1,4 |  |
| heavy truck | 1567 | 1,8 |  |
| trailer truck | 129 | 2,5 |  |
| non-articulated bus | 348 | 1,8 |  |
| articulated bus | 67 | 2,5 |  |
| motorcycle | 252 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is 10,6 \% ( $\omega$ ) of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $v_{s}$, speed of traffic flow) is $60,9 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity (ca) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $59,7 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by -7,2 \% simultaneously.

If so calculate the new capacity utilisation as well.

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $c_{s}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $\mathbf{0 - 1 2 0} \mathbf{~ k m} / \mathrm{h}$ with intervals of $\mathbf{5 - 5} \mathbf{~ k m} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\max }$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $\mathrm{g}=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 6,2 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :---: | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,6 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,69 | $[-]$ friction factor |
| $\mathrm{q}_{s}$ | $=$ | $-2,3$ | $[\%]$ slope (+ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,5 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE* | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 13833 | 1,0 |  |
| light truck | 1056 | 1,4 |  |
| heavy truck | 1756 | 1,8 |  |
| trailer truck | 293 | 2,5 |  |
| non-articulated bus | 439 | 1,8 |  |
| articulated bus | 138 | 2,5 |  |
| motorcycle | 145 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is $11 \%(\omega)$ of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $v_{s}$, speed of traffic flow) is $51,4 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity ( $\mathrm{c}_{\mathrm{a}}$ ) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $47,1 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by $-6,1 \%$ simultaneously.

If so calculate the new capacity utilisation as well.

No.: 10/2020. Name/Neptun: Lopez Lizarraga Julio Cesar/LPMZKT

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $c_{s}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $\mathbf{0 - 1 2 0} \mathbf{~ k m} / \mathrm{h}$ with intervals of $\mathbf{5 - 5} \mathbf{~ k m} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\max }$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $g=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 6,5 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :--- | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,6 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 0,9 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,72 | $[-]$ friction factor |
| $\mathrm{q}_{s}$ | $=$ | $-0,1$ | $[\%]$ slope (+ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,5 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE* | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 8174 | 1,0 |  |
| light truck | 1439 | 1,4 |  |
| heavy truck | 800 | 1,8 |  |
| trailer truck | 265 | 2,5 |  |
| non-articulated bus | 439 | 1,8 |  |
| articulated bus | 125 | 2,5 |  |
| motorcycle | 64 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is $11,4 \%(\omega)$ of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $v_{s}$, speed of traffic flow) is $50,9 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity (ca) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $50 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by $-6,6 \%$ simultaneously.

If so calculate the new capacity utilisation as well.

No.: 11/2020. Name/Neptun: AI Qadri Yahya/R59DCB

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $c_{s}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $\mathbf{0 - 1 2 0} \mathbf{~ k m} / \mathrm{h}$ with intervals of $\mathbf{5 - 5} \mathbf{~ k m} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\max }$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $g=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 6,1 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :---: | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,6 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,66 | $[-]$ friction factor |
| $\mathrm{q}_{s}$ | $=$ | 0,9 | $[\%]$ slope (+ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,6 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE* | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 8722 | 1,0 |  |
| light truck | 3000 | 1,4 |  |
| heavy truck | 925 | 1,8 |  |
| trailer truck | 119 | 2,5 |  |
| non-articulated bus | 158 | 1,8 |  |
| articulated bus | 73 | 2,5 |  |
| motorcycle | 148 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is $11,8 \%(\omega)$ of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $v_{s}$, speed of traffic flow) is $54,7 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity (ca) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $53,5 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by +25 \% simultaneously.

If so calculate the new capacity utilisation as well.

No.: 12/2020. Name/Neptun: Netto de Souza Rodrigo/VPOIW7

Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $c_{s}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $\mathbf{0 - 1 2 0} \mathbf{~ k m} / \mathrm{h}$ with intervals of $\mathbf{5 - 5} \mathbf{~ k m} / \mathbf{h}$. Afterwards calculate the maximum flow (capacity, $q_{\max }$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $\mathrm{g}=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $I_{\text {veh }}$ | $=$ | 5,5 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :---: | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,6 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{\mathrm{r}}$ | $=$ | 1 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,67 | $[-]$ friction factor |
| $\mathrm{q}_{\mathrm{s}}$ | $=$ | 2,7 | $[\%]$ slope (+ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,5 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE* | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 14056 | 1,0 |  |
| light truck | 1903 | 1,4 |  |
| heavy truck | 776 | 1,8 |  |
| trailer truck | 230 | 2,5 |  |
| non-articulated bus | 161 | 1,8 |  |
| articulated bus | 90 | 2,5 |  |
| motorcycle | 300 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is $10,6 \%(\omega)$ of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $v_{s}$, speed of traffic flow) is $61,6 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity ( $\mathrm{c}_{\mathrm{a}}$ ) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $56,2 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by -9,6 \% simultaneously.

If so calculate the new capacity utilisation as well.

## Traffic Flow Exercise 2

## Application of speed-flow relationship

Determine the capacity of a road segment (without intersections) and scale (plan) the required capacity (supply) which fits to the traffic (demand).

1) Determine the capacity (VPHPL, q) of a single lane (and its $c_{s}=80 \%$ spare-value) as a dependent of speed (velocity, v) in a range of $\mathbf{0 - 1 2 0} \mathbf{~ k m} / \mathrm{h}$ with intervals of $\mathbf{5 - 5} \mathbf{~ k m} / \mathrm{h}$. Afterwards calculate the maximum flow (capacity, $q_{\max }$ ) and the optimal speed ( $\mathrm{v}_{\mathrm{opt}}$ ) considering the following average paramameter values ( $g=9,81 \mathrm{~m} / \mathrm{s}^{2}$ ):

| $\mathrm{I}_{\text {veh }}$ | $=$ | 6,4 | $[\mathrm{~m}]$ vehicle length |
| :--- | :--- | :--- | :--- |
| $\mathrm{I}_{\mathrm{s}}$ | $=$ | 0,6 | $[\mathrm{~m}]$ safety dinstance |
| $\mathrm{t}_{r}$ | $=$ | 1,2 | $[\mathrm{~s}]$ reaction time |
| $\varphi$ | $=$ | 0,81 | $[-]$ friction factor |
| $\mathrm{q}_{s}$ | $=$ | $-1,3$ | $[\%]$ slope (+ is uphill, - is downhill) |
| $\alpha$ | $=$ | 0,6 | $[-]$ representing reduced distance headways |

The results of the calculations should be presented in a table as well as in a chart.
2) Scale (plan) the required number of lanes (considering spare capacity as well, $c_{s}$ ) by the use of previuosly determined flow-speed relationship. The traffic volume (demand) can be seen in the following chart. Afterwards calculate the capacity utilisation (CU).

The average daily traffic volume (VPD) of the peak cross-section is the following:

| Vehicle category | Veh/day | PCE $^{*}$ | PCE/day |
| :--- | :---: | :---: | :---: |
| passenger car | 7802 | 1,0 |  |
| light truck | 2951 | 1,4 |  |
| heavy truck | 1550 | 1,8 |  |
| trailer truck | 126 | 2,5 |  |
| non-articulated bus | 428 | 1,8 |  |
| articulated bus | 62 | 2,5 |  |
| motorcycle | 226 | 0,7 |  |

* Passenger car equivalent (PCE).
- the peak hour traffic (PHT) is 9,7 \% ( $\omega$ ) of average daily traffic volume (VPD),
- when peak hour traffic (as the basis of planning) occures on the road the average spacemean speed ( $v_{s}$, speed of traffic flow) is $62,4 \mathrm{~km} / \mathrm{h}$,
- the relation between the number of lanes and the aggregate capacity is the following:

| Number of lanes | Aggregate capacity (ca) |
| :---: | :---: |
| 1 | $100 \%$ |
| 2 | $175 \%$ |
| 3 | $250 \%$ |

3) Examine whether changing (increasing/decreasing) number of lanes is neccessary, if:

- the speed of traffic flow changes to $56,2 \mathrm{~km} / \mathrm{h}$,
- and the PHT changes by $-2,8 \%$ simultaneously.

If so calculate the new capacity utilisation as well.

